

Study Committee B2 OVERHEAD LINES 10914_2022

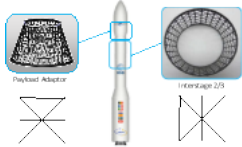
Affordable overhead lines towers compaction using aerospace-borrowed lattices

José Ramón LÓPEZ-BLANCO^a, Pablo RODRÍGUEZ-HERRERÍA^{a,b}, and Carlos GARCÍA-BARRIOS^b

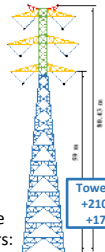
Anisopter Insightful Research^a and Red Eléctrica (Grupo Redeia)^b

Motivation

- Improve social acceptance and reduce project cost.
- OHTL towers compaction reduces visual impact (less corridor width & improved aesthetics), but it may be uneconomical and environmentally unfriendly (more materials & CO₂ release).
- Here, we optimized several aerospace-borrowed grid topologies to reduce material utilization, improve aesthetics, and facilitate the assembly of a new lattice design intended to replace conventional lattice towers:



These earth and space mono-layer lattices that can be generated by repeating the pattern along the axial and hoop directions

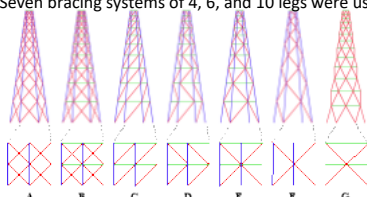


Tower Body
+21000 kg
+17 m Ø

Already applied to OHTLs in Oka river towers one century ago by the Russian engineer and architect V. Shukhov

Tower body geometry

- Seven bracing systems of 4, 6, and 10 legs were used.

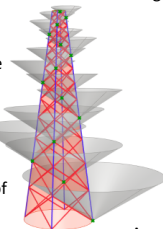


- Selected topologies do not have any diaphragm or sub-bracing to deliberately increase (visual & wind) transparency, simplicity, and aesthetics.

- To assist design, detailing, and assembly, only a single node geometry is required since all diagonals share the same cone angle and the curvature is constant.

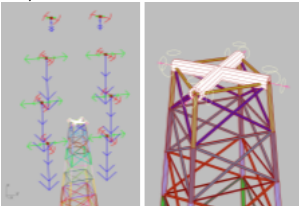
Node positions (green dots) are determined from the intersection points between the cone (grey) from previous level and next leg (blue)

- CHS tubes (EN10210-1 steel S275 and S355) instead of usual angles to further reduce weight and wind drag.



Simulation

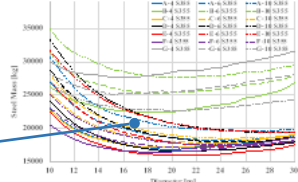
- All practical bracing angles and base diameters were simulated by FEA (2nd order theory)¹. Cross-sections were optimized and conform Eurocode (EN1993.1.1).
- Conductor loads were translated to applied at a single point at tower body top and injected as forces & torques into the structure using rigid and massless bars (white). All load cases in the Spanish norm (ITC-LAT) for a 400 kV DC line were considered.



- Only the highest (59 m) tower body (blue) was utilized to simplify the comparison.
- A similar optimization was performed for an equivalent tubular steel pole design.

Results

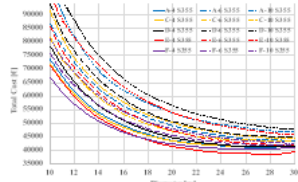
- Minimum mass vs. base diameter and topology:



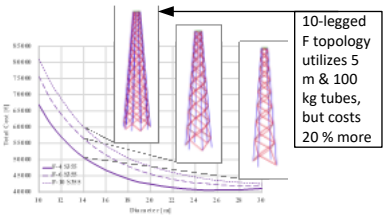
- Steel poles (unstiffened) are 3-4 times heavier:

Type	Mass	Max. Diam.	Thick.	Steel	Found.	Total	
[1]	[kg]	[cm]	[mm]	[t]	[t]	[t]	
0.722	85482	182	2230	31	100781	67862	168643
0.7758	63483	261	1890	35	106419	68824	177243
0.7328	86637	100	2610	35	133421	87082	200503
0.9378	99088	100	2430	43	169441	66767	236207

- Simple cost model elaborated based on tubes and nodes steel (wrt. max. loads), foundations (market data & up-lift²), and land footprint. Cost vs. diameter:

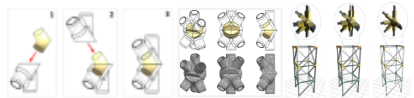


- 4-legged F-topology is the most cost-effective, but bar lengths (10 m) & weights (500 kg) difficult assembly:

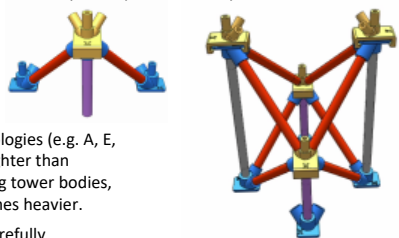


10-legged F topology utilizes 5 m & 100 kg tubes, but costs 20% more

- A versatile rigid connection system based on nodes³ was identified for cost-effective manufacture:



- Assembly concept was virtually validated:



Conclusions

- Whereas several topologies (e.g. A, E, or F) are up to 20% lighter than standard cross-bracing tower bodies, steel poles are 3-4 times heavier.
- A tradeoff must be carefully considered between project expenses, compaction, and manufacturability.
- A standardized connection system based on rigid nodes must be developed to enable cost-effective fabrication.

References

- [1] Preisinger, C. (2013), Linking Structure and Parametric Geometry, Architectural Design, 83: 110-113.
- [2] Working Group 22.09 CIGRE, "Foundation Cost Study" (Electra number 165 April 1996 pages 36-51)
- [3] López Blanco, José Ramón. (2017), Node elements, kits, and methods. European patent No. 3545144 B1.